

Geomechanical simulation of fluid-driven fracture

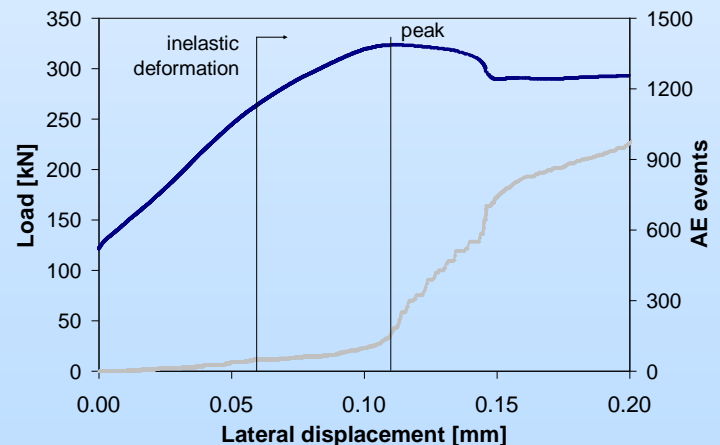
DE-FE0002020

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University of Minnesota**

**U.S. Department of Energy
National Energy Technology Laboratory
Carbon Storage R&D Project Review Meeting
Developing the Technologies and Building the
Infrastructure for CO₂ Storage
August 21-23, 2012**

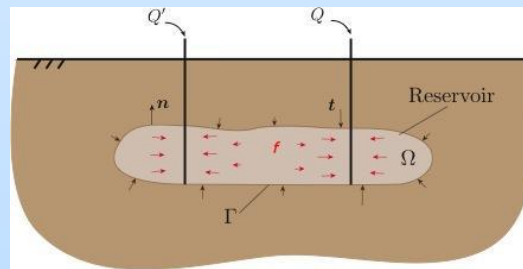
Presentation Outline

- **Benefits statement**
- **Goal, objectives**
- **Technical status: fracture code, experimental results (poro, AE)**
- **Accomplishments**
- **Summary**



Benefit to the Program

- **Goal: develop technologies to predict CO2 storage capacity in geologic formations.**
- **Benefits statement: develop 3D boundary element code & experimental techniques (poro, AE) to simulate fracture in a porous rock; this work contributes to the ability to predict storage and containment.**



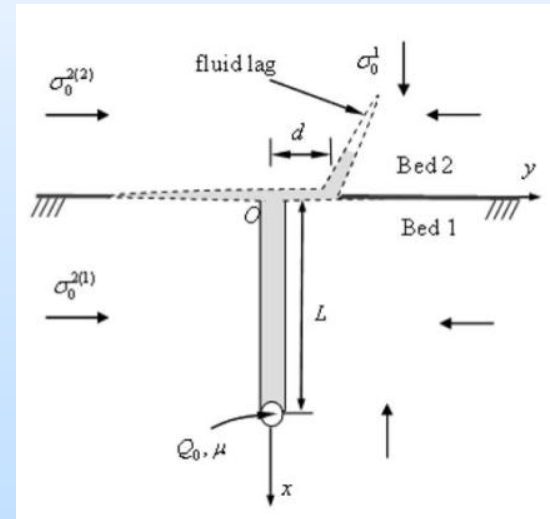
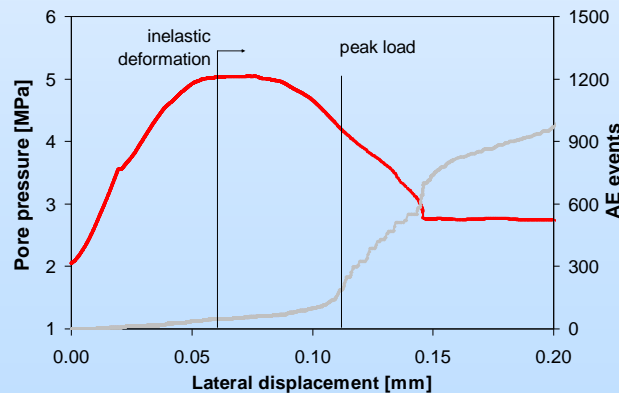
Project Overview: Goals and Objectives

- **Goal: support/train graduate students working on simulation of fracture.**
- **Objectives:**
 - **devise techniques related to laboratory testing of fluid-saturated rock (plane-strain apparatus);**
 - **develop predictive models for the simulation of fracture (3D BEM code);**
 - **establish educational framework for geologic storage issues (poroelastic, exp geomech, BEM).**

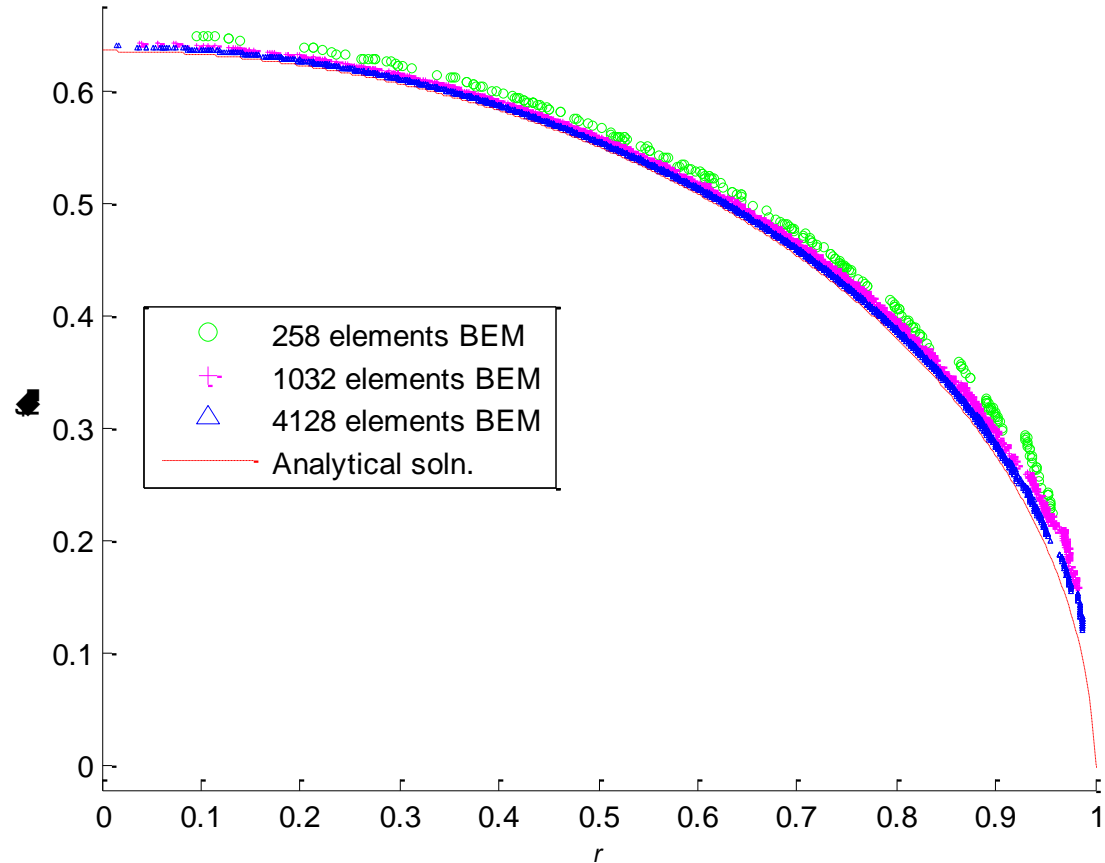
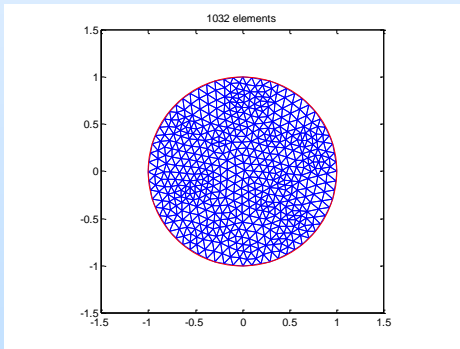
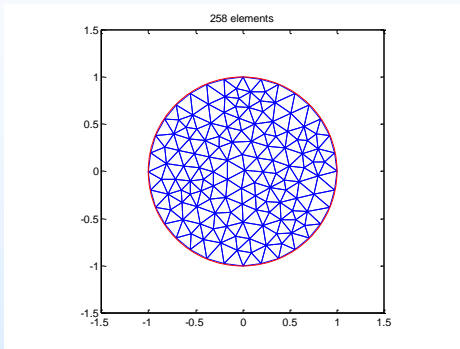


Technical Status

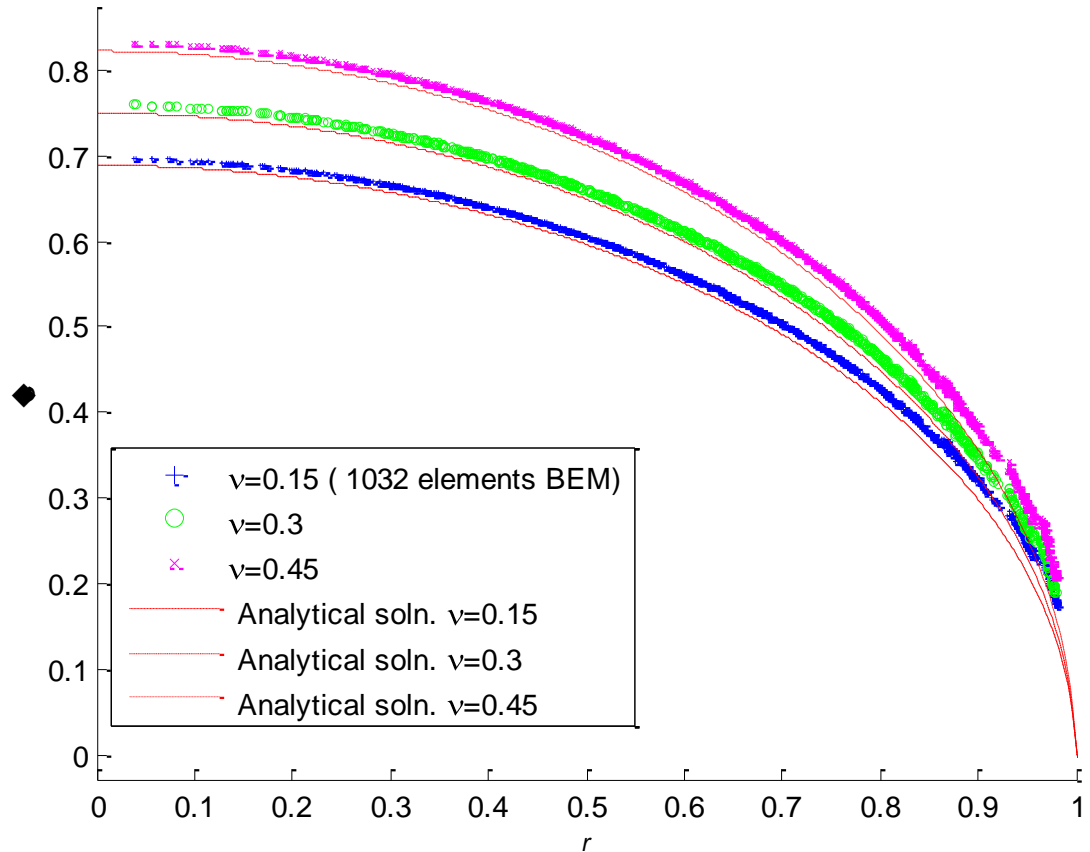
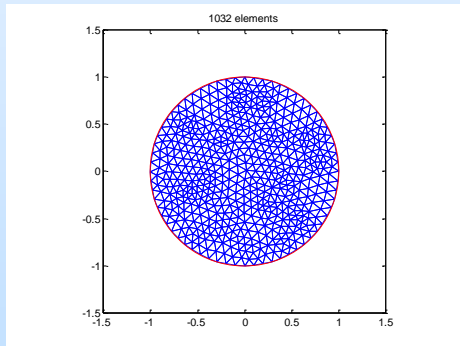
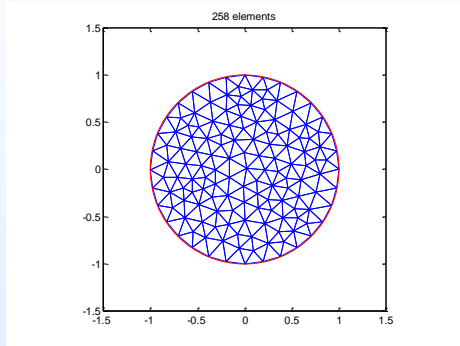
- Fracture code provides crack displacements of fracture (& stresses); develop arbitrarily oriented cracks & boundaries, higher order approximations & crack tip shape functions; arbitrary body force.
- Experimental results.



Penny-shaped crack: mode I

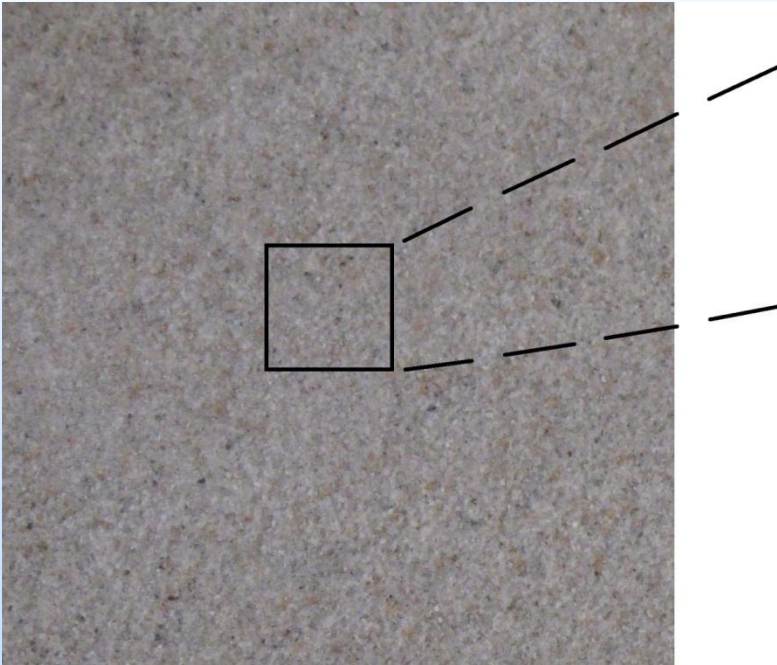


Penny-shaped crack: mode II

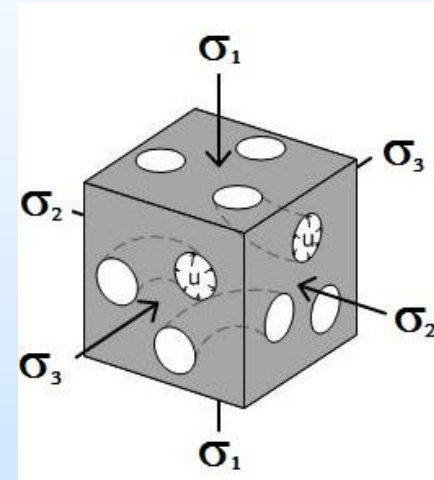


Rock—porous media

Porous sandstone



Representative volume element



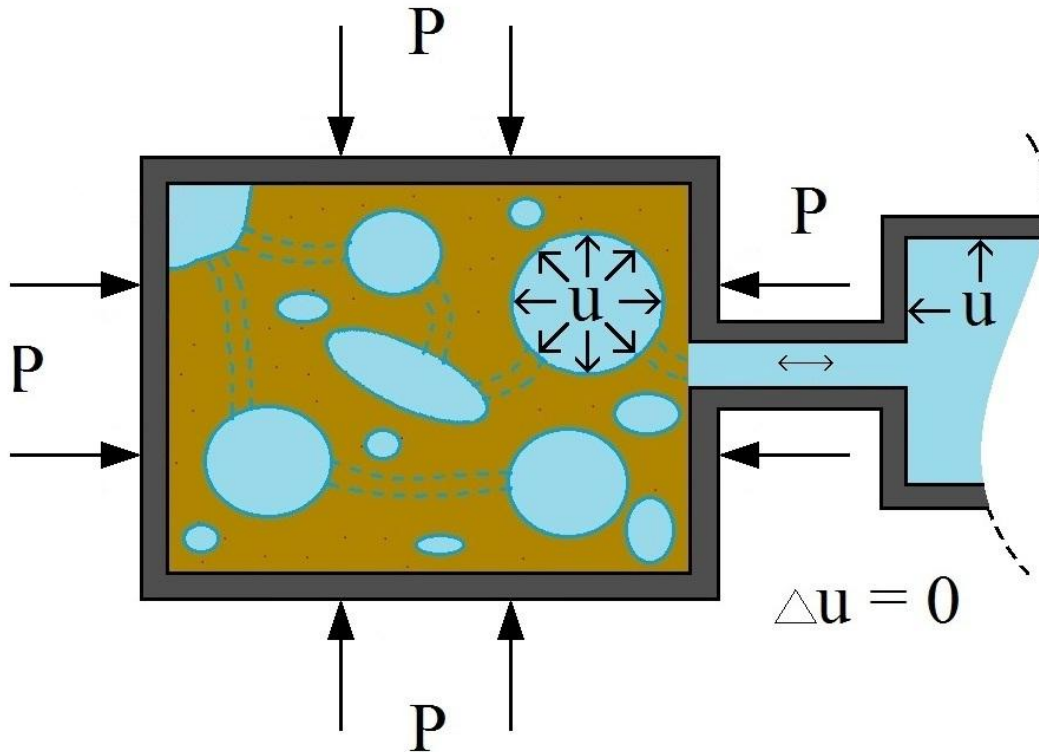
$$\phi = V_\phi / V = \text{porosity}$$

$$P = \sigma_{kk} / 3 = \text{mean stress}$$

$$u = \text{pore pressure}$$



Drained condition



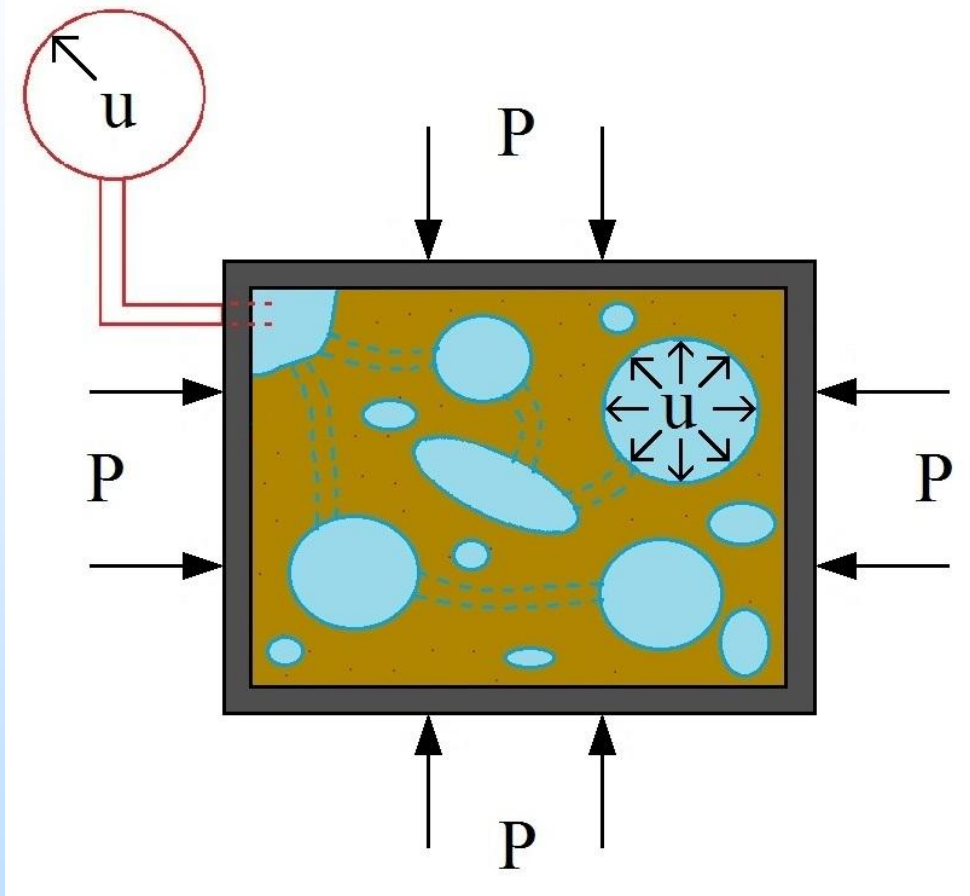
drained: $du = 0$

K = drained bulk modulus

$$K = V \left. \frac{\partial P}{\partial V} \right|_{du=0}$$



Undrained condition



undrained: $dm_f = 0$

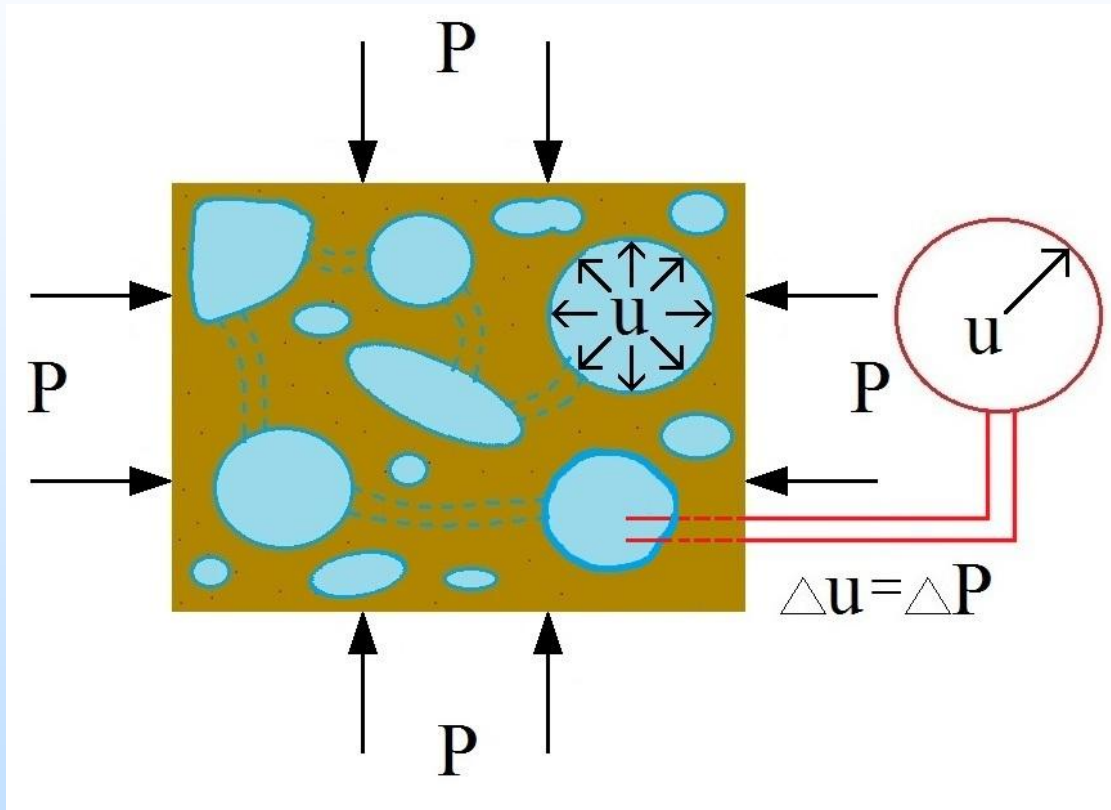
$$K_u = V \left. \frac{\partial P}{\partial V} \right|_{dm_f=0} \quad \text{undrained bulk modulus}$$

$$B = \left. \frac{\partial u}{\partial P} \right|_{dm_f=0} \quad \text{Skempton's coefficient}$$

$$K_f = V_f \frac{\partial u}{\partial V_f} \quad \text{fluid bulk modulus}$$



Unjacketed condition



unjacketed: $dP = du$

unjacketed bulk modulus

$$K_s' = V \left. \frac{\partial u}{\partial V} \right|_{du=dP}$$

unjacketed pore bulk modulus

$$K_s'' = V_\phi \left. \frac{\partial u}{\partial V_\phi} \right|_{du=dP}$$



Equations of poroelasticity

$$\alpha = 1 - \frac{K}{K_s} \quad \text{Biot's coef (1955)}$$

effective stress: $P' = P - \alpha u$

$$K_u = K + \frac{\alpha^2 K}{(1 - \alpha)\alpha + \phi K \left(\frac{1}{K_f} - \frac{1}{K_s} \right)}$$

generalized Gassman equation
(Brown and Korringa 1975)

$$0 < K \leq K_u$$

$$B^{cor} = \frac{1}{\left(\frac{1}{B} \right)_{meas} - \frac{V_L}{V} \frac{K}{\alpha K_f}} = \frac{K_u - K}{\alpha K_u}$$

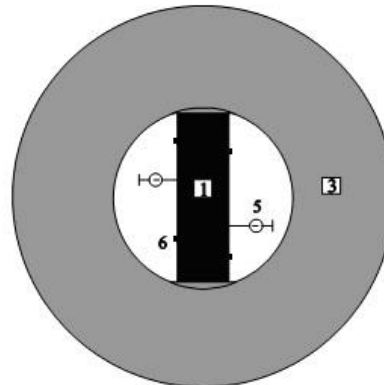
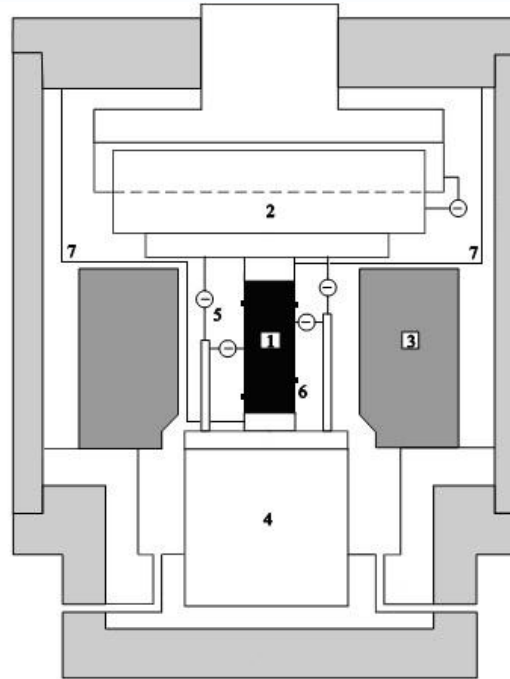
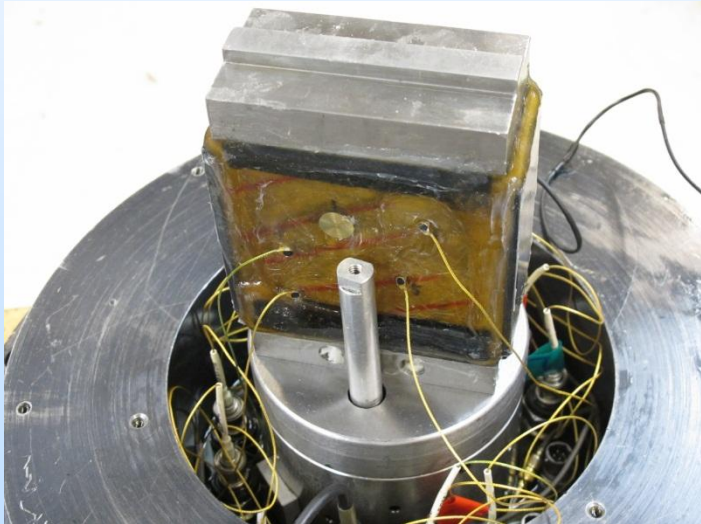
corrected Skempton coefficient
(Bishop 1976)

V_L = volume of fluid in system



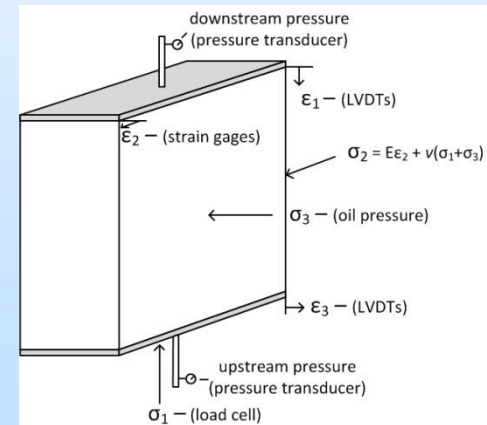
Plane strain testing

University of Minnesota
Plane Strain Apparatus
U.S. Patent 5,063,785



5 LVDTs
8 AE sensors

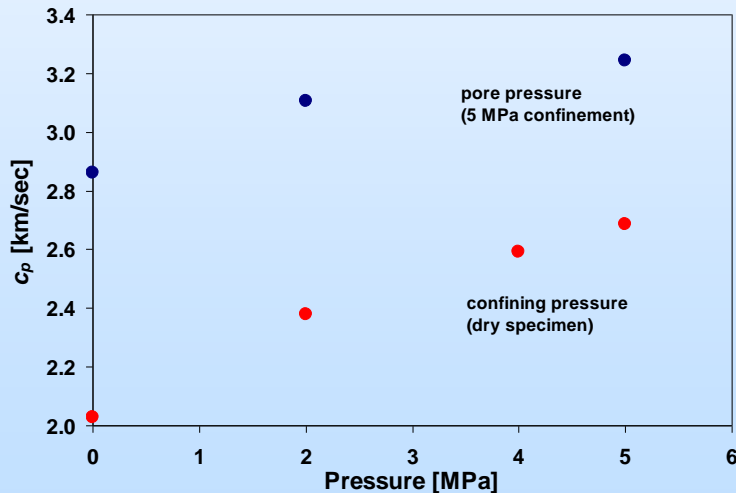
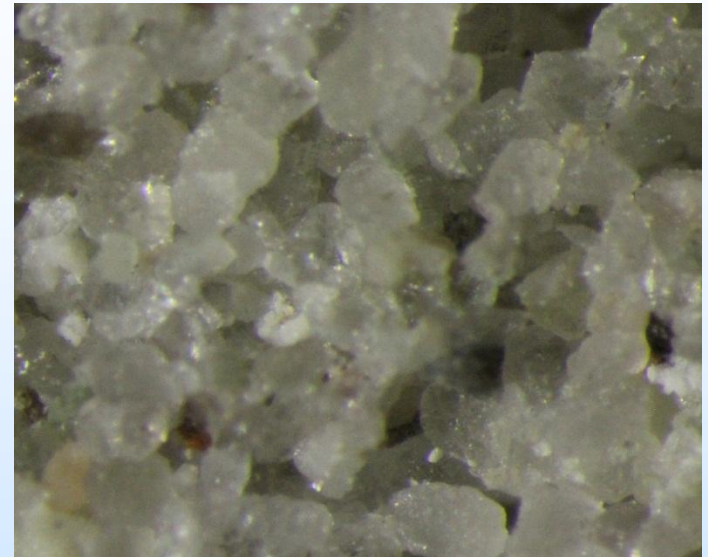
Specimen size:
100 x 86 x 44 mm



Berea sandstone

Slightly anisotropic (5% difference for ultrasonic velocities and 10% in UCS)

Porosity = 23%, permeability = 40 mD (at 5 MPa eff stress), density = 2100 kg/m³, $E = 13-15$ GPa, $\nu = 0.31$



c_p = P-wave velocity
increasing with mean stress
and pore pressure



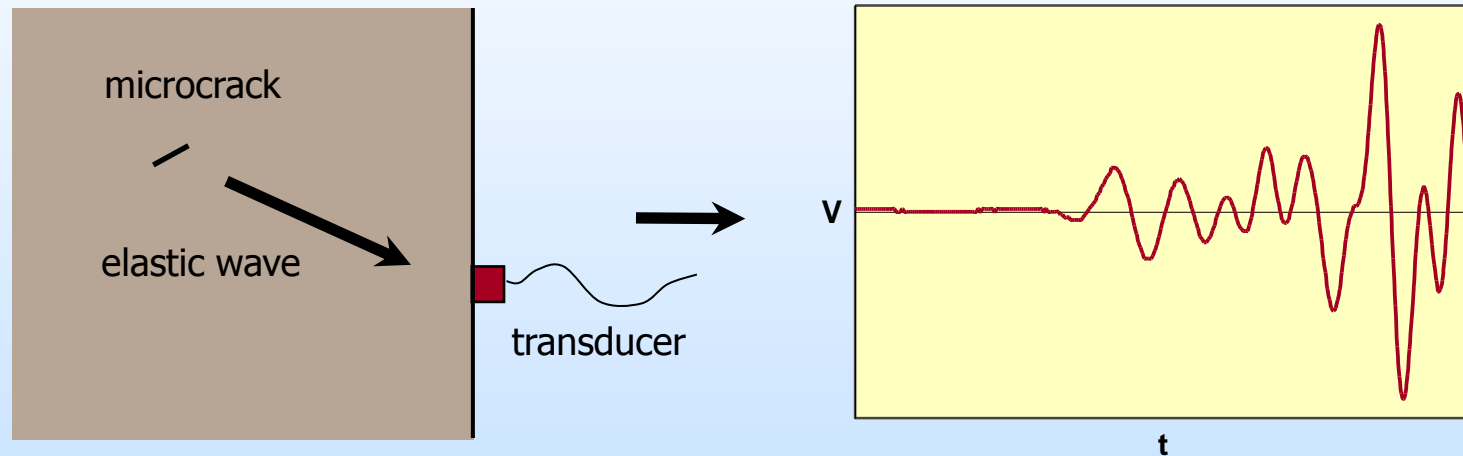
Results

Test #	P [MPa]	u [MPa]	E [GPa]	ν	K, K_u [GPa]	G [GPa]
BxBs-6d	6	0	10.9	0.31	9.6	4.2
BxBs-11d	5	0	10.8	0.32	10.0	4.1
BxBs-2u	8.2	2.8	13.2	0.34	13.8	4.9
BxBs-3u	10	3.8	13.5	0.35	15.0	5.0
BxBs-12u	10	3.4	15.3	0.34	15.9	5.9



Acoustic Emission

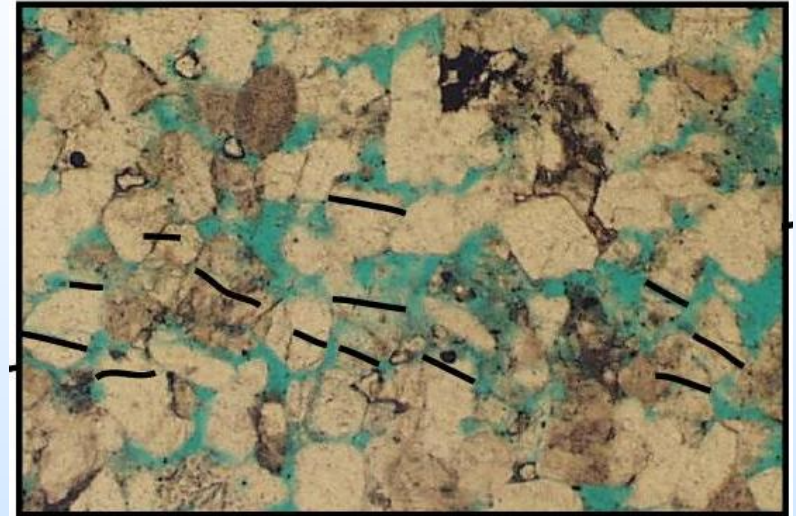
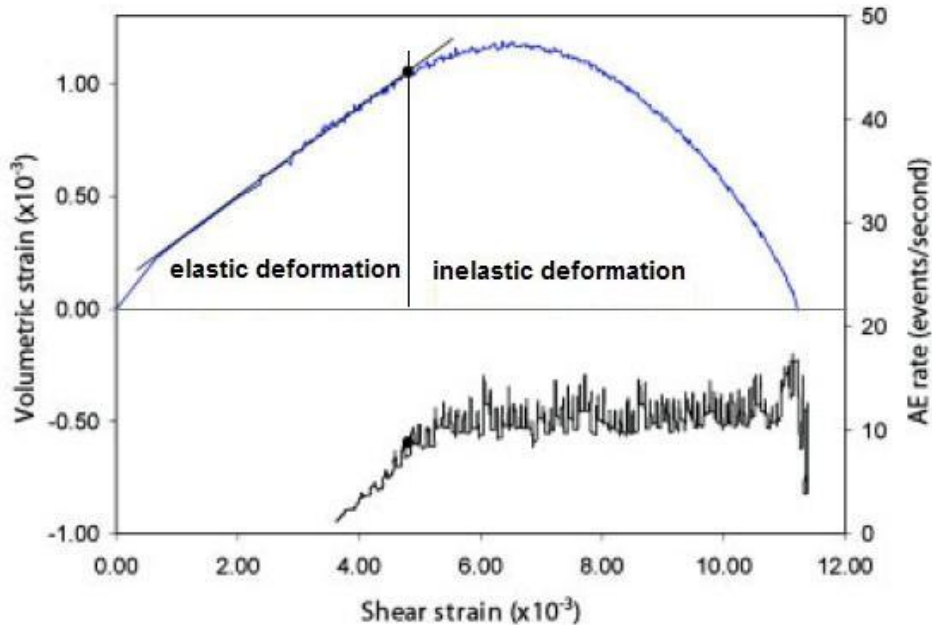
Inelastic response (yielding) of rock is associated with microcracks, which generate elastic waves called acoustic emission (AE).



Transient elastic wave can be recorded by transducers placed on the surface; statistics (rate) and locations (1st arrival) can be studied.



Acoustic Emission



Microphotograph of a fractured rock

In dry rock, increase in AE rate when deformation becomes inelastic.

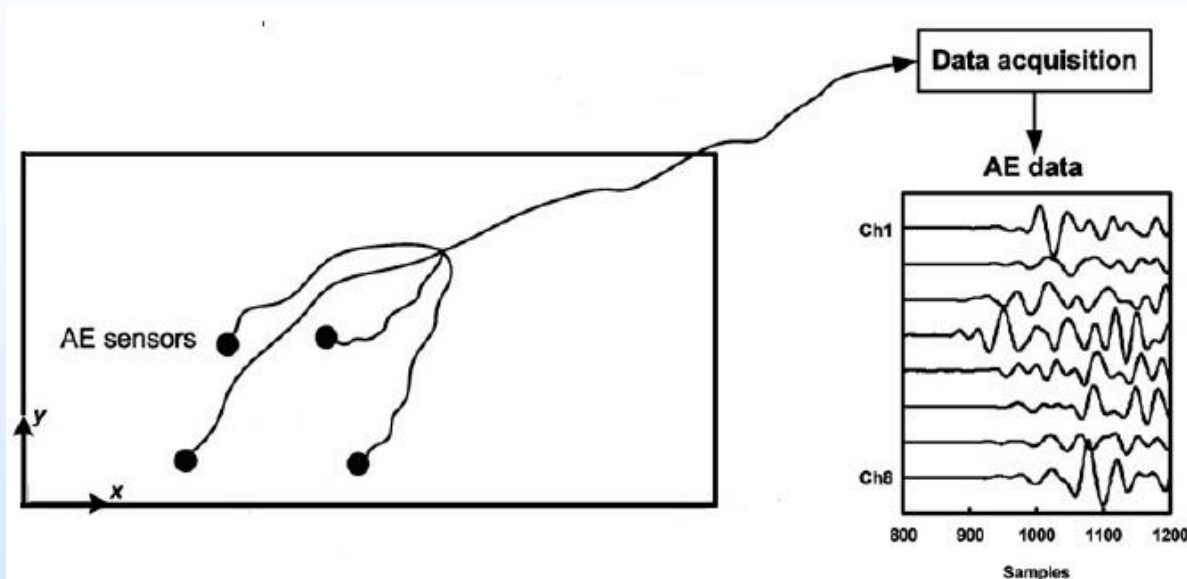
What about liquid-saturated rock?



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AE system



1. AE sensors (0.3-1.8 MHz, 3.6 mm diameter, PA S9225)
2. Preamplifiers (0.1-1.2 MHz filter, 40 dB gain, PA 1220C)
3. Digitizer (LeCroy 6840 or National Instruments 5112)
4. Amplitude threshold trigger



Location of AE

Four unknowns: (x, y, z) and t , event coordinates and time

Know: (x_i, y_i, z_i) sensor coordinates, t_i arrival time at the i^{th} sensor, c_p P-wave velocity

Distance between the source and the i^{th} sensor

$$r_i = c_p \left(t_i - t \right) \pm \varepsilon_i$$

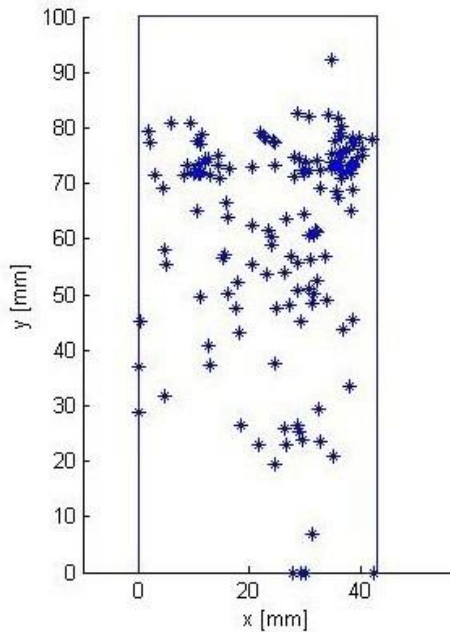
$$r_i = \sqrt{(x_i - x)^2 + (y_i - y)^2 + (z_i - z)^2}$$

Levenberg-Marquardt optimization - minimize I :

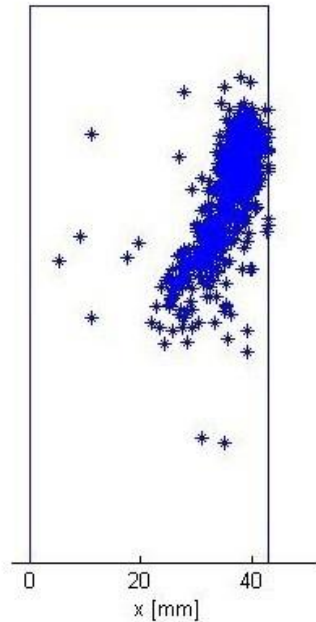
$$I = \sum_{i=1}^N \varepsilon_i^2$$



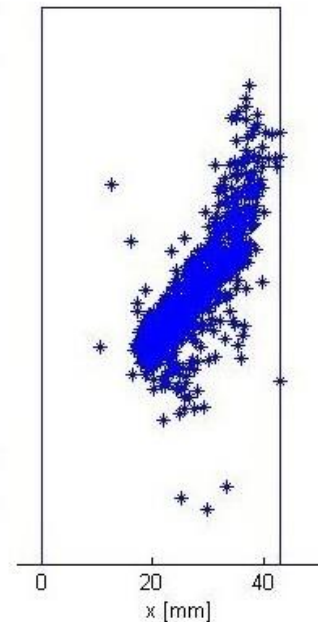
AE locations (dry test)



pre-peak



peak - 90% post-peak



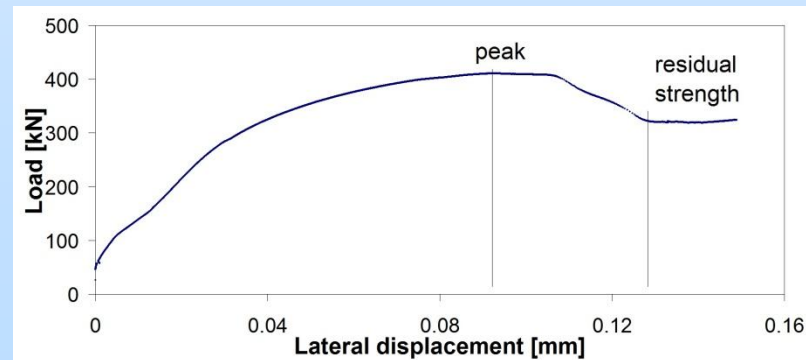
90% post-peak - residual



failed specimen

AE events:

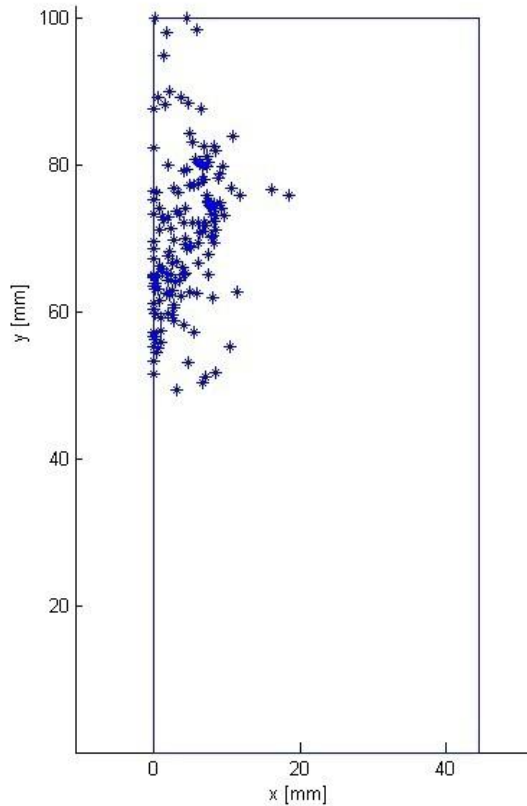
- random before peak
- localized in post-peak



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AE locations (unjacketed)



Location of events



Failed specimen

Possible to detect
AE locations in
liquid saturated rock

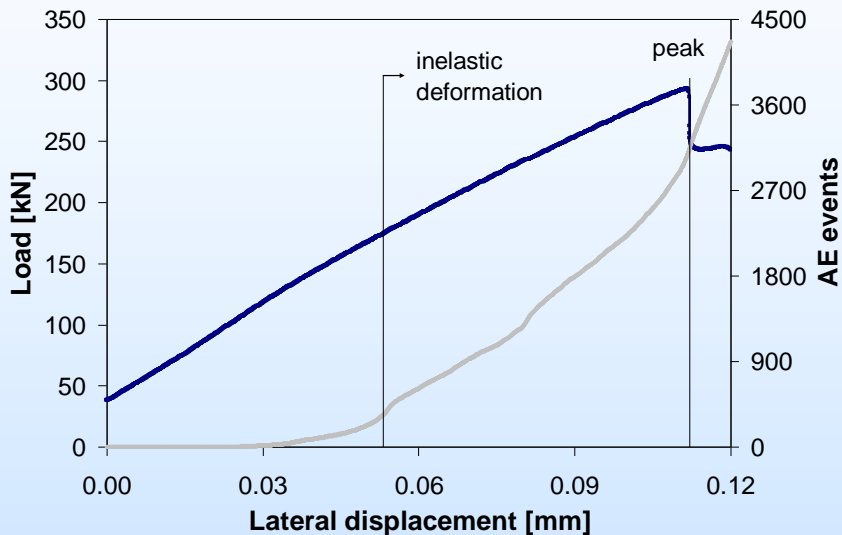
153 events were
located with error
less than 3 mm

Failure mechanism –
axial splitting



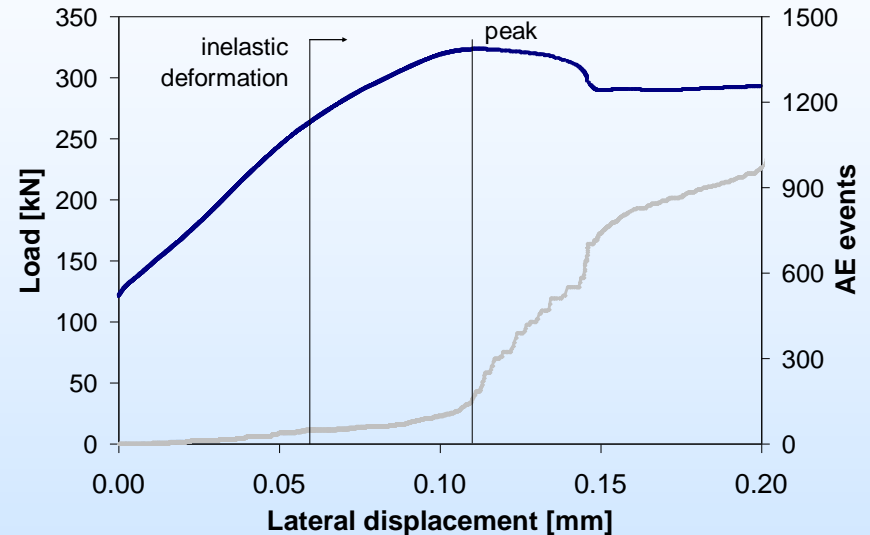
AE rate - load

Drained



Drained:
2700 events in pre-peak

Undrained

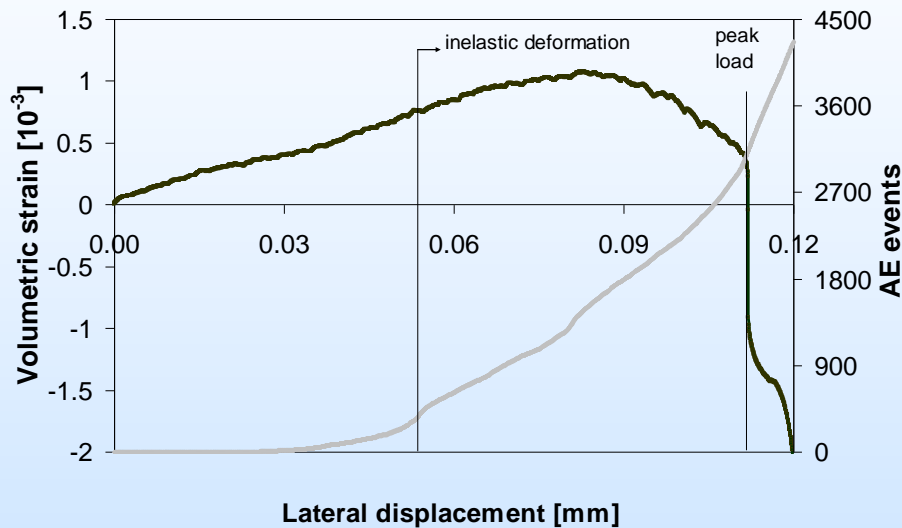


Undrained:
170 events in pre-peak



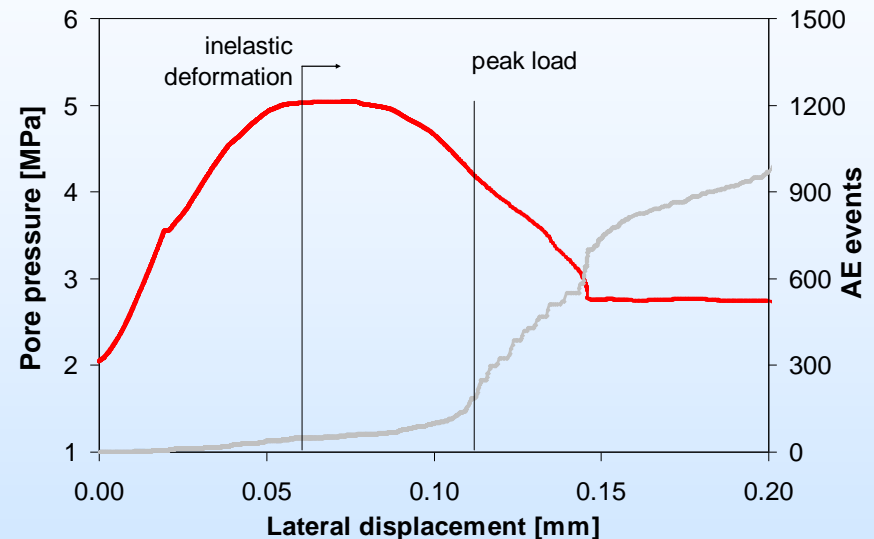
AE rate - deformation

Drained



Drained (rates):
inelastic 130 events/min
post-peak 410 events/min

Undrained

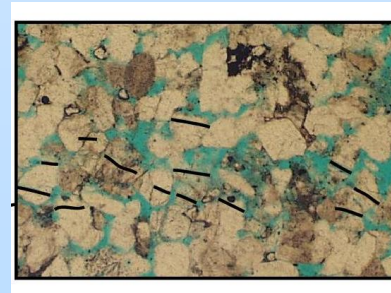


Undrained (rates):
inelastic 3 events/min
post-peak 210 events/min



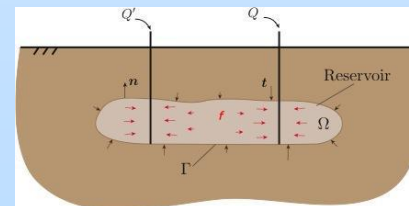
Accomplishments to Date

- BEM code to simulate crack propagation; needed to assess storage/containment.
- Poroelastic parameters from drained and undrained plane-strain compression; needed to predict reservoir response.
- AE rates found to be different under drained and undrained conditions; *rock's tendency to dilate delayed under undrained condition*. To assess reservoir response, inelastic behavior must be understood.

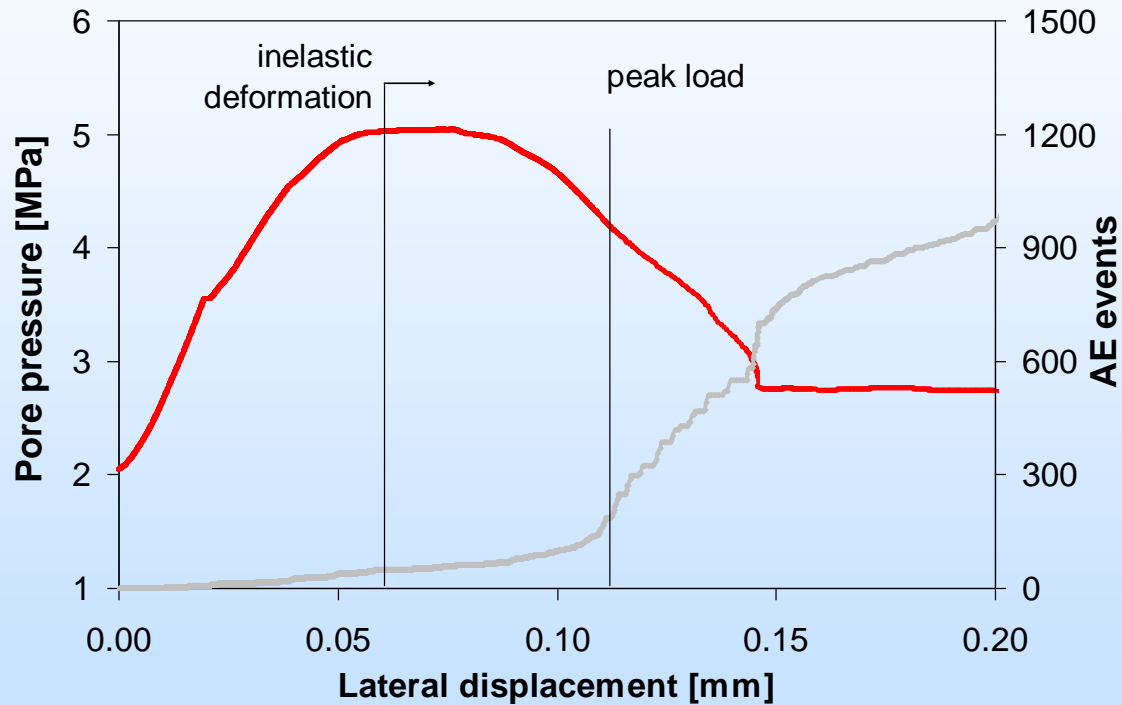


Summary

- **Key Findings: 3D BEM fracture code with body forces; poroelastic parameters from plane strain compression testing.**
- **Lessons Learned: saturation critical.**
- **Future Plans: assessment of risks related to fracturing of the reservoir and the caprock; heterogeneity of rock mass; body force (pore pressure gradient induced) a significant feature.**



Appendix



Organization “Chart”

- **J.F. Labuz, PI: experimentalist, with two patents; fracture and strength of rock, acoustic emission. E. Detournay, co-PI: poroelasticity, hydraulic fracturing. S. Mogilevskaya, co-PI: applied mathematician, boundary integral methods, especially modeling fracture propagation.**
- **R. Makhnenko, D. Nikolski: Ph.D. students; A. Pyatigorets: Ph.D., partial support; J. Meyer: M.S., partial support.**



Gantt Chart

Activities	Time (1 block = 2 months)											
	Year 1				Year 2				Year 3			
Task 1.0 Project management												
Task 2.0 Experiments												
2.1 System calibration												
2.2.1 Undrained testing												
2.2.2 Drained testing												
2.3 AE/damage assessment												
Task 3.0 Numerical modeling												
3.1 Two-D BEM												
3.2 Three-D BEM												
3.3 Fluid coupling												
Task 4.0 Course development												
4.1 Experimental mechanics												
4.2 Poro/thermal elasticity												
4.3 Boundary element modeling												

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